

**Heterogeneity of Subsurface Pore Distribution: Characterization based on Pressure and Tracer Responses to Identify Undiscovered Permeable Structures in Reservoirs**

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**Key Points:**

- This study proposed and validated a concept for quantifying the heterogeneity of subsurface pore distribution.
- The concept combines conventional pressure transient and tracer testing, assuming diffusion and advection–dispersion problems.
- This concept provides a useful scale to quantify undiscovered permeable structures in exploring and developing reservoirs.

## **Abstract**

Pressure transient and tracer testing are conventional methods employed to investigate physical properties associated with fluid flow and/or storage in subsurface reservoirs or aquifers. These methods have been adopted independently to investigate from different physical aspects. Here, to quantify the heterogeneity of pore distributions containing subsurface fluid, a novel concept, which combines the pressure and tracer concentration responses obtained during pressure transient and tracer tests, respectively, has been proposed and validated. Herein, the key parameter is the difference between the apertures of the equivalent planar fractures estimated from the pressure and tracer concentration responses. In particular, this difference is attributed to the fact that the pressure and tracer concentration responses obey different physical mechanisms, diffusion and advection–dispersion problems, respectively, that generate dissimilar responses to the heterogeneity of pore distribution. The concept was successfully validated using laboratory experiments and reservoir simulations conducted at multiple scales. As observed, the apparent pore volume estimated from the pressure responses tended to be larger than the actual value owing to the delay in pressure responses during propagation through the pore. By quantifying the existence of undiscovered permeable structures in a reservoir simulation model, the proposed concept provides an insightful guide for successful decision-making in explorational and developmental geothermal projects. Furthermore, the concept provides a scale for assessing the accuracy of a reservoir simulation model in expressing an actual heterogeneous permeable structure.

## **Plain Language Summary**

Conventionally, the physical properties of subsurface structures such as reservoirs or aquifers involving flow and/or storage of fluids (e.g., water, oil, and gas) are investigated using two testing methods: pressure transient test and tracer test. The pressure transient test measures the pressure variations occurring during production and/or injection of fluid using wells, whereas the tracer test monitors the variations in the concentration of tracers (e.g., ionized substances and stable isotopes) at production wells and springs. This study combined the pressure and tracer testing methods to propose a novel concept and validated it to quantify the complexity of the pore distribution of a reservoir or aquifer containing branches and dead ends. This concept utilizes the difference in physical mechanisms that pressure and tracer concentration obey,

diffusion and advection–dispersion problems, respectively. The concept was successfully validated using laboratory experiments and computer simulations. Overall, the concept can provide an insightful guide for successful decision-making in explorational and developmental geothermal projects by quantifying the existence of undiscovered permeable structures in reservoir simulation models. Furthermore, the proposed concept provides a scale for reservoir simulation models to accurately express an actual complex reservoir or aquifer.

## **Keywords**

heterogeneity, pressure transient testing, tracer testing, reservoir modeling, exploration and development, geothermal project

## **1 Introduction**

Adequate understanding of subsurface fluid flow under heterogeneous pore distribution is key to successfully resolve numerous geoscientific and engineering problems. As such, heterogeneity is generated in a wide range of scales, including an outcrop of several tens to hundreds of meters associated with a fracture network (Bisdom et al., 2016; Zuo et al., 2019) and a fault intersecting a field spanning several kilometers (Erdogmus et al., 2006; Goko, 2000; Rae et al., 2004). A reliable and direct approach for investigating fluid flow under such conditions involves the observation of in situ responses such as temporal variations in pressure and solute concentration at one or more observational points, while generating artificial flow at a scale corresponding to the given problems. Pressure transient and tracer testing methods are popular conventional methods that can estimate the physical parameters, productivity, performance, and mass transfer associated with fluid flow and/or storage in subsurface reservoirs or aquifers at a field scale.

Pressure transient testing measures the pressure responses during production and/or injection of fluid to estimate several parameters such as transmissivity, storativity, skin factor, and wellbore storage constant by modeling fluid flow in a reservoir surrounding the wellbore according to Darcy's law (Agarwal et al., 1970; Bourdet et al., 1983; Theis, 1935; van Everdingen & Hurst, 1949; van Everdingen, 1953). In the oil and gas industry, several key parameters such as reservoir size, geometry, total pore volume, and mechanisms supporting reservoir pressure are estimated for project feasibility studies, considering essential information

on recoverable reserves and forecasted decline in production (Horne, 1995; Houzé et al., 2012). Historically, the geothermal industry has applied several techniques developed in the oil and gas industry. In the geothermal industry, pressure transient testing employs a reservoir simulation model to estimate the possible production and reinjection rates for continuous sustainable power generation across several decades (DiPippo, 2016; Grant & Bixley, 2011; Zarrouk & McLean, 2019). Assuming that type curves obey several extended models to express heterogeneity, such as the leaky aquifer (Hantush, 1956), dual-porosity model (Barenblatt et al., 1960; Warren & Root, 1963), linear and bilinear flows associated with a fracture (Cinco-Ley et al., 1978; Gringarten et al., 1974), MINC model (Pruess & Narasimhan, 1982, 1985), fractional dimension models (Acuna & Yortsos, 1995; Chang & Yortsos, 1990), and other flexible numerical models using application software such as AWTAS (O'Sullivan et al., 2005) and Saphir (Houzé et al., 2012), reservoir engineers have been able to interpret and estimate the pressure responses under diverse conditions. The models commonly used for analyzing pressure responses obey the diffusion problems for both quasi-incompressible and compressible fluids by adopting pseudo-pressure (Al-Hussainy et al., 1966).

Tracer testing uses flowing wells to inject a tracer into an injection well and measure the tracer concentration responses at the production wells. Referring to the tracer concentration responses and obeying advection–dispersion problems, the direction and velocity of subsurface fluid flow along with multiple properties such as porosity, dispersibility, hydrostratigraphy, and heat transfer characterization have been successfully determined (Grove & Beeten, 1971; Güven et al., 1986; Hall, 1993; Klepikova et al., 2016; Leaf et al., 2012; Mackay et al., 1986; Reimus et al., 2003). For several decades, a variety of tracers have been adopted, e.g., solid particles, ionized substances, stable isotopes, radioactive substances, organic dyes, gases, and fluorocarbons, as well as water temperature (Davis et al., 1980). In principle, tracers must satisfy several conditions such as detectivity, chemical stability, negligible absorption, and absence in a natural state. Additionally, reactive tracers have been adopted for advanced techniques (Adams & Davis, 1991; Davis et al., 2000; Lemke et al., 2013). Comprehensively, tracer testing is applied across a diverse range of subsurface fluid fields such as the oil and gas (Cockin et al., 2000; Patidar et al., 2022; Tomich et al., 1973) and geothermal (Chrysikopoulos, 1993; Rose et al., 2001; Sanjuan et al., 2006) industries. In tracer testing, investigating and characterizing the heterogeneity of the above-mentioned physical properties are crucial for successful modeling and

forecasting of subsurface fluid flow. Accordingly, several scholars have attempted to overcome these problems by adopting novel techniques such as multilevel–multitracer testing methods and equipment, reactive and nonreactive tracers (e.g., deoxyribonucleic acid molecules), and stochastic simulations (Ptak et al., 2004).

This study developed and validated a concept to quantify the heterogeneity of pore distribution associated with subsurface fluid flow based on the novel perspective of combining the pressure and tracer concentration responses obtained during conventional pressure transient and tracer tests, respectively. As discussed earlier, the pressure and tracer concentration responses obey the diffusion and advection–dispersion problems, respectively. This difference in the physical mechanisms yields dissimilar responses to the heterogeneity of the pore distribution. The pressure variations occurring during a pressure transient test propagates throughout a reservoir, regardless of pore geometry, whereas the fluid flow containing a tracer is only generated at a portion of the total pore volume along the flow paths, which avoids the branches and dead ends in a quasi-steady state. Thus, the visible pore volume obtained using the tracer test is limited, whereas the pressure transient test can be employed to investigate the total pore volume. If both visible volumes coincide, the actual pore distribution can be interpreted as homogeneously planar without branches and dead ends. Based on this concept, we defined a scale to quantify the heterogeneity of the pore distribution from a particular aspect: the deviation of the actual pore distribution from a homogeneously planar distribution. This study aimed to elucidate this concept at multiples scales, followed by the validation through laboratory experiments and numerical reservoir simulations. Ultimately, a potential application of the proposed concept has been described for quantifying the existence of permeable structures that remain undiscovered during the exploration and development of reservoirs. Furthermore, this concept enables the assessment of a reservoir simulation model in accurately expressing an actual permeable structure.

## **2 Concept**

### **2.1 Fracture Network with Branches and Dead Ends**

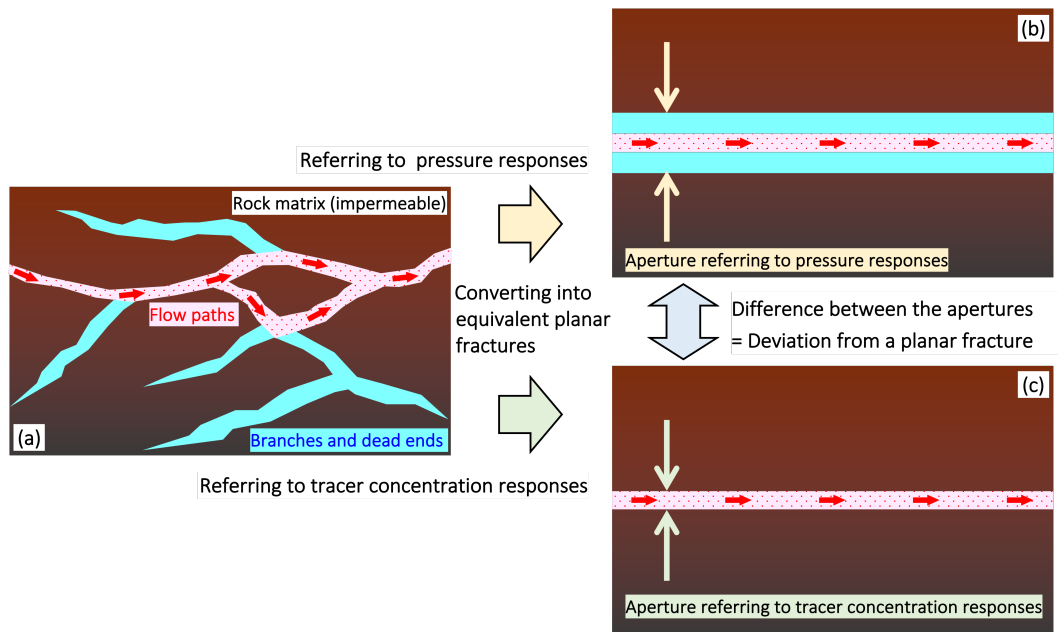
Let us consider two schematic examples at distinct scales. In the first case, we consider a portion of a naturally fractured water-dominated reservoir at a relatively small scale of several

134 meters to several tens of meters (Figure 1a). Generally, permeable fracture networks contain  
135 numerous branches and dead ends that generate heterogeneity in the pore distribution (Bisdom et  
136 al., 2016; Zuo et al., 2019). For simplicity, the rock matrix was assumed as fully impermeable.  
137 Upon performing a pressure transient test using wells intersecting the reservoir, the pressure  
138 variation obeying a diffusion problem propagates throughout the fracture network, regardless of  
139 the branches and dead ends. By analyzing the pressure responses at the flowing or monitoring  
140 wells at the field scale, the total pore volume including the volume of branches and dead ends is  
141 quantified and estimated as a storativity value (Horne, 1995; Houzé et al., 2012; Zarrouk &  
142 McLean, 2019). Structures equivalent to the branches and dead ends are potentially generated  
143 along the fracture surface via nonuniform aperture distribution (Pyrak-Nolte et al, 1988;  
144 Sawayama et al., 2021) forming an enclosed space with a narrow entrance. A fault core itself  
145 may branch or anastomose, sometimes with branching subsidiary faults (Faulkner et al., 2010).

146 In general, tracer tests are performed under stable conditions with approximately constant  
147 flow rates after an adequate period of time has elapsed since the start of production and  
148 reinjection (Fukuda et al., 2005; Kumagai & Kitao, 2000; Rose et al., 2001; Sanjuan et al., 2006),  
149 which allows the assumption of a quasi-steady state in a reservoir. Under this condition, the flow  
150 paths of the tracer are determined predominantly along the continuous pathways from a  
151 reinjection well to production wells, avoiding branches and dead ends (Figure 1a). At a  
152 sufficiently smaller time-scale compared with the molecular diffusion toward each branch, tracer  
153 concentration responses at the production wells represent only the limited pore volume  
154 contributing to the generation of the tracer flow paths. This pore volume can be estimated by  
155 analyzing the tracer concentration responses to determine several conventional parameters such  
156 as total recovery and residence time (Grant & Bixley, 2011). Alternatively, a more direct and  
157 sophisticated approach involves simulating tracer concentration responses by developing a  
158 reservoir simulation model that has been successfully applied in multiple fields (Egert et al.,  
159 2020; Nakao et al., 2007; Ponte et al., 2009).

160 Estimated based on pressure responses, storativity can be regarded as the product of the  
161 total compressibility and aperture of an equivalent planar fracture (i.e., porosity–thickness  
162 product for a porous layer), as demonstrated by Grant and Bixley (2011) for Ohaaki field. Based  
163 on the pressure responses, the equivalent planar fracture considers the total pore volume,  
164 including the volume of branches and dead ends. Moreover, another equivalent planar fracture

can be determined according to the tracer concentration responses that represent only a portion of the total pore volume covered by a tracer (Figure 1b–c). In case the fracture network contains no branches or dead ends, both apertures coincide to indicate a single planar fracture. Thus, the difference between the apertures of the equivalent planar fractures can be regarded as a scale quantifying the deviation from the homogeneously planar pore distribution, which signifies the heterogeneity of the pore distribution. As portrayed in Figure 1a, flow paths may occasionally include minor bypasses, and accordingly, such bypasses will not contribute toward generating a deviation from the planar pore distribution. Notably, major bypasses that cannot be neglected based on pressure and/or tracer concentration responses are interpreted as multiple planar pore distributions. In the exemplary case of Ohaaki field (Grant & Bixley, 2011), a porosity–thickness product of 300 m was estimated based on the pressure interference at a monitoring well. This is because a single planar reservoir was assumed for analyzing the pressure interference, regardless of the actual three-dimensional reservoir structure (Mroczek et al., 2016).



**Figure 1.** Schematic of a fracture network forming a reservoir and equivalent planar fractures. (a) Heterogeneous pore distribution generated by fracture network contains continuous flow paths as well as branches and dead ends. (b) Aperture of an equivalent planar fracture determined referring to pressure responses represents the total pore volume, because pressure variation propagates throughout the total pore volume. (c) Aperture determined referring to tracer concentration responses represents only a portion of the total pore volume that generates flow

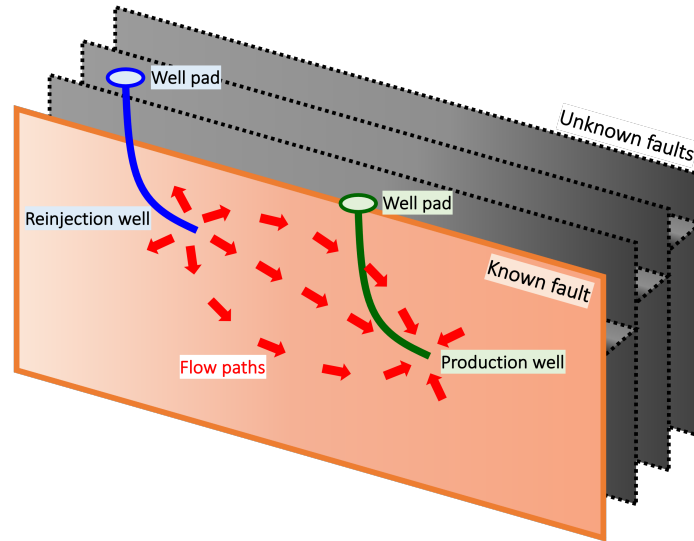
paths. The difference between the apertures quantifies the deviation of the fracture network from a planar fracture.

## 2.2 Faults Distributed in Parallel

The second case assumes a relatively large scale of several kilometers to consider the total prospect. Let us consider a water-dominated isothermal reservoir with heterogeneous pore distribution generated by parallel-distributed steep faults with stepwise displacement, as described by Bense et al. (2013) and Cole et al. (2005). All the faults were connected to each other through horizontal permeable structures. As described in Figure 2, one of the faults is known through an ongoing exploration, whereas the others remain unknown. Accordingly, pressure transient and tracer tests were performed using production and reinjection wells intersecting a known fault. The pressure variation caused by production and reinjection propagates throughout the reservoir, including the unknown faults. A tracer is injected into the reinjected fluid under quasi-steady-state conditions after a sufficient period of time has elapsed since the start of production and reinjection. In particular, the tracer flow paths are strongly controlled by the fluid flow from the reinjection to the production well through the known fault, because both wells intersect only the known fault. Thus, the tracer concentration response at the production well represents only a portion of the total pore volume generated by a known fault. At this stage of ongoing exploration, reservoir engineers have attempted to analyze the pressure and tracer concentration responses by assuming only a known fault. However, the aperture of an equivalent planar fracture determined from the pressure response is larger than that evaluated using the tracer concentration response.

As discussed earlier, the difference between the apertures of equivalent planar fractures with respect to the pressure and tracer concentration responses indicates a deviation from the planar pore distribution. On a large scale, a difference between the apertures indicates the existence of unknown faults, as observed in the second case. Thus, this difference can act as a potential indicator to quantify the unexplored permeable structures, which will provide us an insightful guide for performing prospective explorations in future. As long as the pressure variation obeys diffusion problems, this indicator is valid for fractured reservoirs as well as for both porous and fractured porous reservoirs.





**Figure 2.** Schematic of parallel-distributed steep faults. Production and reinjection wells are drilled directionally and intersect the known fault only if the other three faults are unknown. All faults are connected through horizontal permeable structures.

### 3 Laboratory Experiments

The concept described in Section 2 is validated herein. For the first case involving the relatively small scale, we simulated the pressure and tracer concentration responses using laboratory experiments. The experiments were performed assuming a quasi-Hagen–Poiseuille flow in the urethane tube. Based on the experimental results, the original Hagen–Poiseuille flow was modified to account for the additional pressure loss. We aimed to derive a mathematically common diffusion equation with respect to pressure, which will describe the mass conservation in a tube and reservoir as both quasi-Hagen–Poiseuille and Darcy flows contain a common mathematical expression. Overall, the apparent and actual inner diameters of the tube corresponded to the apertures of the equivalent planar fractures discussed in Section 2.

#### 3.1 Apparatus

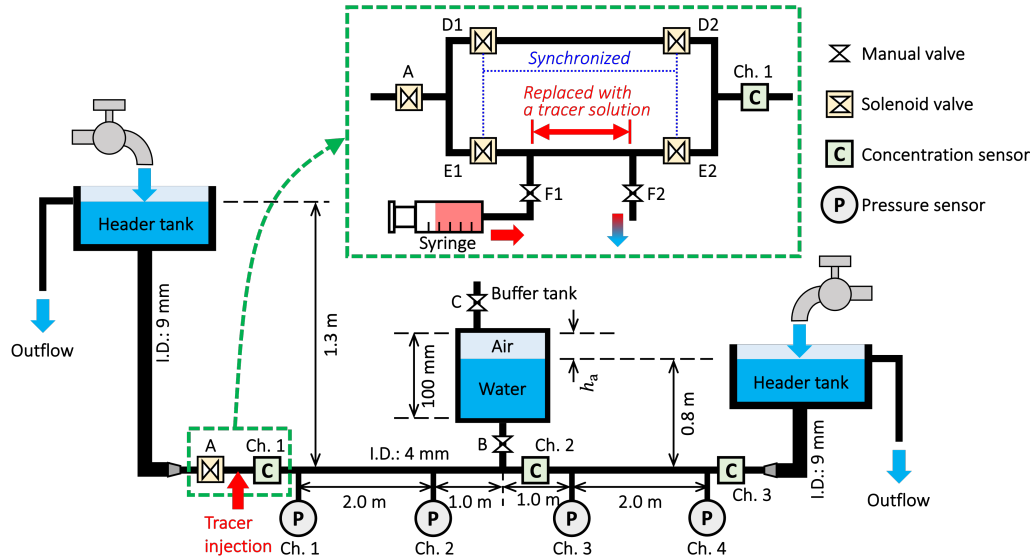
A schematic of the experimental apparatus assembled under atmospheric pressure and temperature conditions is illustrated in Figure 3. This apparatus was used to generate a flow of tap water under a controlled, constant pressure gradient in a horizontal urethane of 4 mm inner diameter. The pressure gradient was controlled by adjusting the upstream and downstream header tank levels, which were connected to the urethane tube through silicon tubes with a larger

inner diameter of 9 mm to reduce pressure loss. In addition, four diaphragm pressure sensors (PK025SA506, Fuji Controls Co., Ltd.) corresponding to Channels 1–4 were placed along the urethane tube. The pressure sensors produced an output analogue voltage ranging from 0.2–4.6 V, corresponding to a pressure variation of 0.0–25.0 kPaG, recorded at intervals of 1 ms using a high-speed data logger. Prior to the experiments, the voltage values were calibrated and validated to reduce the individual differences between the sensors within 0.024 V (0.13 kPa). Eventually, the flow was generated by opening Valve A placed upstream of the urethane tube.

At the midpoint of the horizontal urethane tube, a cylindrical buffer tank was connected (Figure 3). This tank buffered the transient pressure variations in the horizontal tube. The thickness of air  $h_a$  in the buffer tank can be adjusted by leveling the buffer tank via closing Valves A, E1, and E2 and opening Valves B, C, D1, and D2. As observed, the water level in the buffer tank corresponded to that in the downstream header tank. Valve C was closed, except for adjusting the air thickness in the buffer tank. As illustrated in Figure 1, we intended to mimic a branch and a dead end by connecting the buffer tank. In addition, the effective volume of the mimicked branch and dead end filled with water could be varied by adjusting the air thickness in the buffer tank. Let us assume that the mixture of water and air in the buffer tank exhibits bulk compressibility  $\bar{c}$  and a total volume  $V$ . If a variation in this volume of mixture caused by compression is equal to that in the effective volume  $V'$  filled with water, the relationship  $V'/V = \bar{c}/c_w$  is satisfied, where  $c_w$  denotes the compressibility of water. The buffer tank can be disconnected by closing Valve B, which mimics a planar fracture without branches or dead ends.

A solution of the tracer Ponceau 4R can be injected into the horizontal tube (Figure 3). At the instant of closing Valves E1 and E2 and opening Valves F1 and F2, the water in the interval between Valves E1 and E2 was replaced with a solution injected with a syringe through Valve F1. Subsequently, the water originally stored in the interval was ejected through Valve F2. The volumetric capacity of the interval was estimated as 3.87 mL based on the inner diameter of the tube (4 mm), length (300 mm), and spacing (8 mm in total) of the connectors. Approximately 10 mL of the solution was injected by referring to the scale on the syringe, and the ejection of red-colored water through Valve F2 was observed. After introducing the tracer solution, it can be injected into the water flowing in the tube by closing Valves D1 and D2 and simultaneously opening Valves E1 and E2. As such, the motions of the four solenoid valves were electrically

synchronized, and three concentration sensors for the tracer were placed near both ends and the midpoint of the horizontal urethane tube.



**Figure 3.** Schematic of experimental apparatus. At the midpoint of the horizontal urethane tube, a buffer tank with an inner diameter of 93 mm and height of 100 mm was connected through another urethane tube with an inner diameter of 4 mm and length of 1.5 m. Water levels in the buffer tank and that in the downstream header tank were maintained identical. The precise distances of the pressure sensors placed in Channels 2–4 from that in Channel 1 were 2.008, 4.024, and 6.032 m, respectively, including the spacings in the connectors. The precise distances of the concentration sensors located in Channels 2 and 3 from that in Channel 1 were 3.325 and 6.238 m, respectively. The distance between the pressure and concentration sensors in Channel 1 was 0.132 m. During the experiments, the ambient pressure and temperature measured in the laboratory were 996.9 hPa and 28.8 °C, respectively.

We designed in-house concentration sensors based on those developed by Takaki et al. (2015). In these sensors, green light with a peak wavelength of 520–525 nm from a light-emitting diode (DiCUNO) was transmitted through the urethane tube and received by a phototransistor (NJL7502L, Nisshinbo Micro Devices Inc.) with a peak sensitivity wavelength of 560 nm. As the peak absorbance wavelength of the tracer Ponceau 4R was 508 nm (Bevziuk et al., 2017), the voltage between a resistor connected to the emitter of the phototransistor varied sensitively depending on the tracer concentration in the water flowing in the tube. The

relationship between the concentration  $C_n$  [wt%] for Channel  $n$  and voltage  $V$  [V] was calibrated for each sensor using reference solutions prepared with multiple concentrations. The relationships were summarized using the following empirical expressions (Table 1, Figure 4):

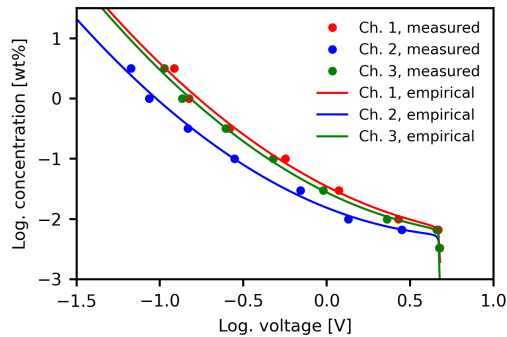
$$\log C_n = a_n + b_n \log V + c_n (\log V)^2 + \frac{d_n}{\log V - \log V_{0n}}, \quad (1)$$

where  $a_n$ ,  $b_n$ ,  $c_n$ , and  $d_n$  denote constants, and  $V_{0n}$  [V] indicates the voltage of pure water. The constants were determined using the least-squares method implemented.

**Table 1**

*Constants for Equation 1*

$n$	$a_n$	$b_n$	$c_n$	$d_n$	$\log V_{0n}$
1	-1.45790	-1.40794	0.634143	0.00105583	0.681784
2	-1.81174	-1.09609	0.661727	0.000914718	0.678973
3	-1.55684	-1.35305	0.679273	0.000940129	0.680879



**Figure 4.** Relationship between tracer concentration and output voltage for each concentration sensor. Empirical relationships obey Equation 1 with the constants summarized in Table 1.

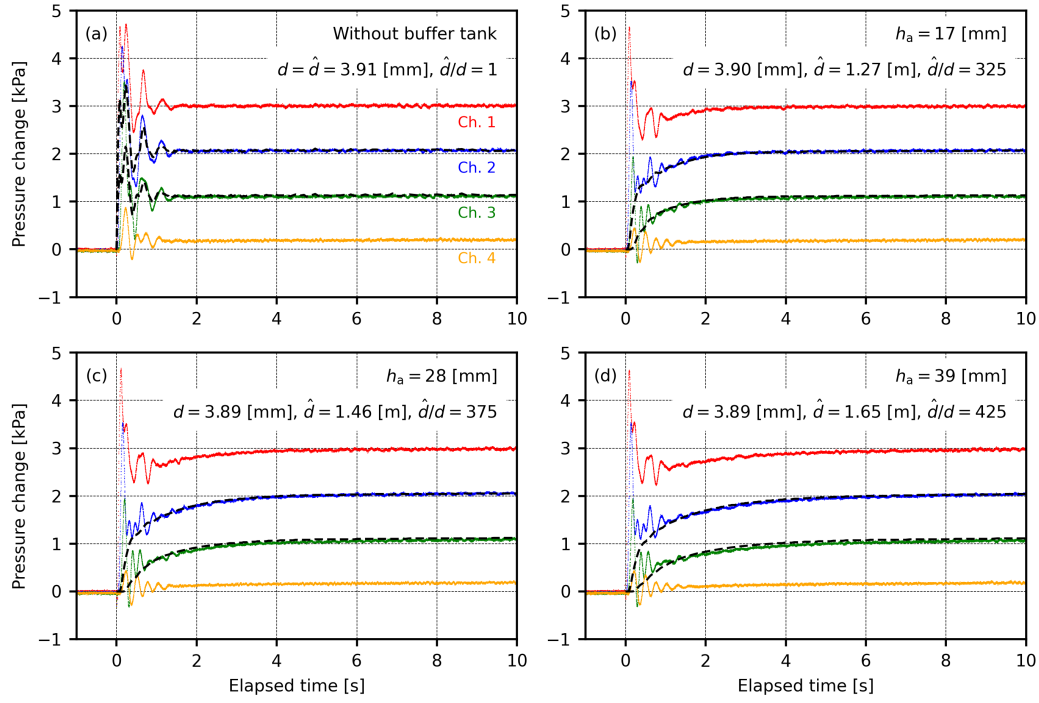
### 3.2 Conditions and Results

Four experiments were performed using various conditions of the buffer tank. In the first experiment, the buffer tank was disconnected, whereas the remaining experiments were performed by connecting the buffer tank with air thicknesses of 17, 28, and 39 mm. The water

levels of the upstream and downstream header tanks were set at 1.3 and 0.8 m, respectively, above the horizontal urethane tube (Figure 3). Under steady-state conditions with this pressure gradient, the volumetric flow rate in the horizontal tube was determined in advance by measuring the overflow from the downstream header tank as  $141.26 \text{ mL min}^{-1}$  at no water supply to the downstream header tank. This value represents the average of ten measurements with a standard deviation of 1.6%. Prior to each experiment, the water originally stored in the interval between Valves F1 and F2 was replaced with a tracer solution (0.5 wt %).

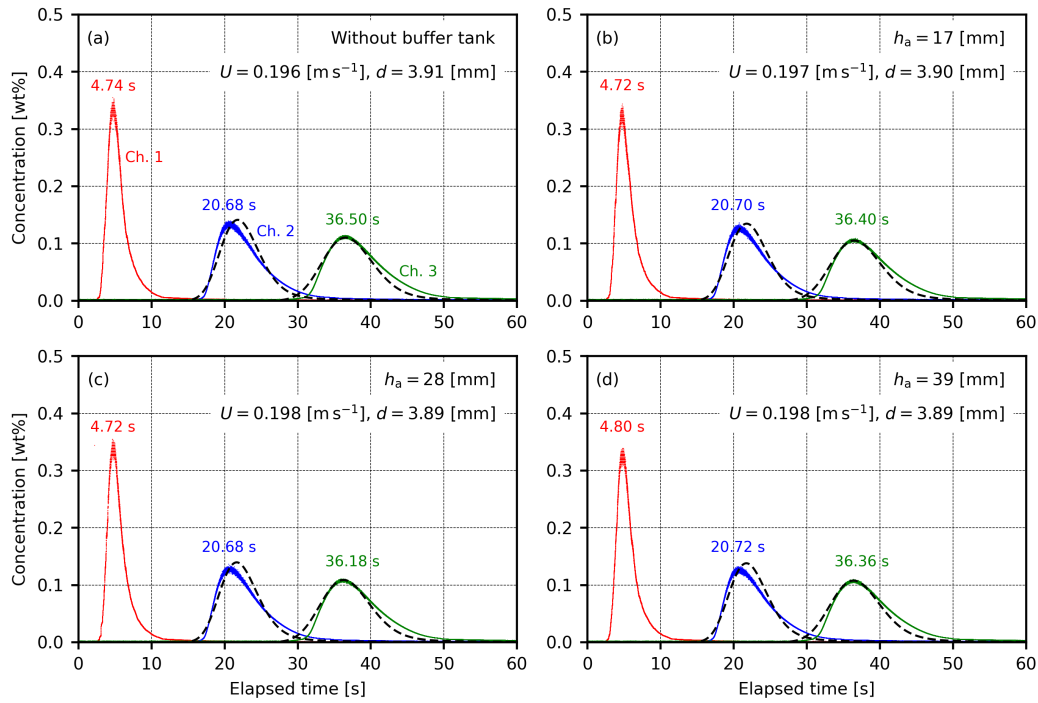
The experiments were performed using a common procedure. In the initial state, Valves A, C, E1, E2, F1, and F2 were closed, whereas Valves D1 and D2 were open. The state of Valve B varied depending on the experiment. First, the transient pressure response was measured after opening Valve A. The pressure monitored at each sensor converged to a constant value after a few tens of seconds at most after opening Valve A, which indicated a steady state. After 110 s of opening Valve A, the tracer was injected by opening Valves E1 and E2, and closing Valves D1 and D2. The tracer concentration response was measured for 90 s after tracer injection. The precise time of opening Valve A and injecting the tracer could be determined from the pressure data that recorded noise caused by the motions of the valves.

The pressure responses during the four experiments are presented in Figure 5. All responses initially exhibited noise with relatively high frequencies, which was caused by the motion of Valve A during opening. After the noise ceased, monotonous variations were observed, during which the pressure variations differed with the buffer tank conditions. Without the buffer tank (Figure 5a), the pressure response in each channel varied steeply. As the buffer tank was connected (Figure 5b–d), the pressure responses became more gradual with increasing air thickness  $h_a$  in the buffer tank. The pressure responses for all the experiments ultimately reached a common steady state, independent of the buffer tank condition. The tracer concentration responses are presented in Figure 6. All responses exhibited a steep variation at the upstream point (Channel 1), followed by gradual variations at the downstream points (Channels 2 and 3) because of mechanical dispersion. Remarkably, none of the tracer concentration responses exhibited an apparent dependence on the buffer tank condition.



**Figure 5.** Pressure responses after opening Valve A for four buffer tank conditions. (a) Buffer tank was disconnected. (b)–(d) Air thickness  $h_a$  in the buffer tank was modified from 17 to 39 mm. Red, blue, green, and orange plots indicate experimental observations at the sensors for Channels 1–4, respectively. Broken lines indicate numerical solutions of Equation 2. Estimated values of apparent tube inner diameter  $\hat{d}$  and the ratio of that to the actual inner diameter  $\hat{d}/d$  are

noted for each experiment. Values of actual inner diameter  $d$  were estimated from tracer concentration responses (Figure 6).



**Figure 6.** Tracer concentration responses after tracer injection in four buffer tank conditions. (a) Disconnected buffer tank. (b)–(d) Air thickness  $h_a$  in buffer tank was modified from 17 to 39 mm. Red, blue, and green plots indicate experimental observations at sensors for Channels 1–3, respectively. Broken lines indicate numerical solutions of Equation 3. The observed peak time for each sensor as well as the estimated flow velocity  $U$  and tube inner diameter  $d$  are presented for each experiment.

### 3.3 Discussion

The concept described in Section 2 is experimentally validated herein. Assuming a quasi-Hagen–Poiseuille flow of water with compressibility  $c_w$  and viscosity  $\mu$  in the horizontal urethane tube with the actual inner diameter  $d$ , we obtain the following diffusion equation with respect to the pressure  $P$  depending on time  $t$  and distance  $x$  along the tube based on mass conservation:

$$c_w \hat{d}^2 \frac{\partial P}{\partial t} = a \frac{d^4}{32\mu} \frac{\partial^2 P}{\partial x^2}, \quad (2)$$

where  $a$  and  $\hat{d}$  are the modification factor and apparent inner diameter of the tube, respectively. The apparent inner diameter  $\hat{d}$  of the tube can vary from the actual inner diameter  $d$  depending on the condition of the buffer tank. Considering the flow in a single tube,  $\hat{d} = d$ . The modification factor  $a$  is factored to modify the original Hagen–Poiseuille flow to correspond to the experimental results, including the additional pressure loss. Furthermore, we obtain the advection–dispersion equation with respect to the tracer concentration  $C$  in the horizontal tube as follows:

$$\frac{\partial C}{\partial t} + U \frac{\partial C}{\partial x} = D \frac{\partial^2 C}{\partial x^2}, \quad (3)$$

where,  $U$  and  $D$  are the flow velocity and dispersion coefficient, respectively. Under steady-state conditions in the experiments, the flow velocity  $U$  is determined as  $U = 4Q/(\pi d^2)$ , where  $Q$  denotes the volumetric flow rate.

Assuming a single tube, we initially determined the flow velocity and inner diameter of the horizontal urethane tube according to the tracer concentration responses (Figure 6) and the volumetric flow rate measured prior to the experiments. The flow velocity was determined from the observed peak instant and the distance for each concentration sensor using linear regression. The inner diameter of the tube was determined from the flow velocity and volumetric flow rate. As summarized in Figure 6, the flow velocity and inner diameter of the tube were independent of the buffer tank condition. The inner diameter of the tube was estimated at approximately 4 mm, which is consistent with the specifications provided by the manufacturer. In particular, the connection to the buffer tank did not affect advection and dispersion in the horizontal tube.

The broken lines in Figure 6 represent the numerical solutions of Equation 3 using the estimated flow velocity value  $U$  for each experiment. The value of the dispersion coefficient  $D$  was commonly assumed as  $6.0 \times 10^{-3} \text{ m}^2 \text{ s}^{-1}$  for correspondence between the simulations and observations. We adopted a combined numerical technique of the constrained interpolation profile method (Yabe et al., 1991; Yabe & Aoki, 1991) to reduce numerical dispersion for the advection term and the conventional explicit finite difference method for the dispersion term. The grid sizes for temporal and spatial discretization were  $1.0 \times 10^{-4} \text{ s}$  and  $1.0 \times 10^{-2} \text{ m}$ ,



respectively. For the upstream boundary condition, we assumed the temporal variations observed at the sensor located in Channel 1. In addition, an outflow boundary was assumed for the downstream boundary condition at the location of the sensor in Channel 3. The numerical solutions validated the consistency of the experimental results with the theoretical solution of the one-dimensional advection–dispersion problem (Equation 3).

Subsequently, we interpreted the pressure responses (Figure 5) in terms of the one-dimensional diffusion problem (Equation 2). The broken lines in Figure 5 indicate the numerical solutions to Equation 2, which were obtained assuming the actual inner diameter  $d$  of the tube based on the tracer concentration response observed in each experiment (Figure 6). The compressibility  $c_w$  and viscosity  $\mu$  of water were assumed as  $5.1210 \times 10^{-10} \text{ [Pa}^{-1}\text{]}$  and  $8.1823 \times 10^{-4} \text{ [Pa s]}$ , respectively, referring to the measured ambient pressure and temperature as well as the empirical equations developed by the International Association for the Properties of Water and Steam (IAPWS) (IAPWS, 1997, 2007). The volumetric flow rate estimated by assuming these parameter values and the Hagen–Poiseuille flow was less than that measured prior to the experiments ( $141.26 \text{ mL min}^{-1}$ ), implying additional pressure loss. To compensate this inconsistency, we adopted a quasi-Hagen–Poiseuille flow containing a modification factor  $a$ , ranging from 0.72–0.74, as expressed in Equation 2. The additional pressure loss was potentially generated by the connectors for the pressure sensors and buffer tank as well as the loop-shaped horizontal urethane tube with a diameter of  $\sim 0.3 \text{ m}$ . The values of the Reynolds number determined from the conditions of these experiments ranged from approximately 930–940, which are less than the critical value of 1800 (Landau & Lifshitz, 1987) for the transition between laminar and turbulent flows. We adopted the conventional implicit finite difference method to solve Equation 2 numerically. To simulate a pressure response for 10 s, the temporal grid size expanded exponentially from  $1.0 \times 10^{-9}$  to  $1.5 \times 10^{-3} \text{ s}$ , and the spatial grid size was uniform at  $1.0 \times 10^{-2} \text{ m}$ . The boundaries with temporally varying pressures were assumed according to the variations observed at the sensors located in Channels 1 and 4.

Under the above conditions, the pressure variations observed at the sensors placed in Channels 2 and 3 during each experiment were simulated in series, and the ratio of the tube inner diameter  $\hat{d}/d$  was altered to ensure correspondence between the simulations and observations. As depicted in Figure 5, the simulations with the best-fit value of  $\hat{d}/d$  successfully reproduced

all the experimental observations, including that without the buffer tank ( $\hat{d}/d = 1$ ). Depending on the effective volume of the water-filled buffer tank, the connection of the buffer tank, such as that in the experiments, is equivalent to assuming an apparent tube inner diameter  $\hat{d}$  several hundred times larger than the actual value  $d$ . This implies that the difference between the apparent and actual inner diameters quantifies the deviation from a single tube. Compared with the diffusion equation with respect to reservoir pressure (Zarrouk & McLean, 2019), the factors  $c_w \hat{d}^2$  and  $ad^4/32\mu$  in Equation 2 correspond to the storativity and transmissivity of a reservoir, respectively. Based on the experimental results, we conclude that an increase in the volume of branches and dead ends (Figure 1) mimicked using the buffer tank (Figure 3) can be quantitatively indicated by the apparent increase in storativity, which is defined as the product of the total compressibility and aperture of an equivalent planar fracture. Assuming a total compressibility value based on the realistic values for fluids and formations, the increase in storativity can be interpreted as an increase in the aperture of an equivalent planar fracture. Alternatively, the aperture of an equivalent planar fracture can be independently estimated from the tracer concentration responses, such as that in the experiments (Figure 6), which only represents a portion of the total pore volume and is unaffected by the branches and dead ends. Thus, based on the pressure and tracer concentration responses, the deviation from the planar pore distribution containing no branches or dead ends can be quantified in terms of the difference between the apertures of equivalent planar fractures, as described in Section 2. Further quantitative discussions regarding the pore volume are presented in Section 5.

#### 4 Reservoir Simulations

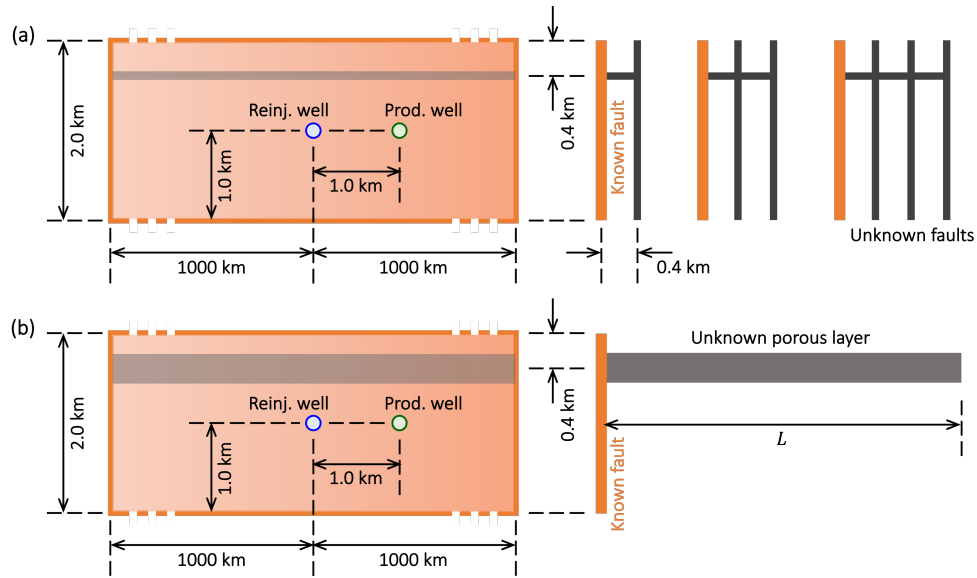
The concept described in Section 2 was validated at a relatively large scale using numerical reservoir simulations. As discussed earlier, we assumed a known vertical fault and unknown permeable structures such as faults or a porous layer. The unknown permeable structures were connected to the known fault. The pressure transient and tracer tests were performed using production and reinjection wells that intersected only the known fault. Accordingly, the pressure and tracer concentration responses were investigated under several conditions of unknown permeable structures.

#### 4.1 Conditions

The two models used for the numerical reservoir simulations are presented in Figure 7. Both models assumed a known vertical fault with a vertical dimension of 2.0 km. Assuming an extremely large horizontal dimension of 2000 km, we intended to place infinite-acting boundary conditions. The first model assumed that the vertical unknown faults were distributed parallel to the known fault (Figure 7a), and the vertical and horizontal dimensions of the unknown vertical faults were identical to those of the known faults. The spacing between the known and unknown faults was uniform at 0.4 km, and all faults were connected at 0.4 km from the upper boundary through a horizontal permeable structure. The second model assumed an unknown horizontal porous layer (Figure 7b). The dimensions of the unknown porous layer in the direction perpendicular to the known fault ranged several kilometers, whereas that parallel to the known fault was 2000 km, as assumed for the known fault. The unknown porous layer intersected the known fault at 0.4 km from the upper boundary. The production and reinjection wells intersected the known fault at the vertical middle points with a spacing interval of 1.0 km. The production and reinjection flow rates were constant at 200 and 160 t h<sup>-1</sup>, respectively. Subsequently, a tracer was injected into the reinjected fluid after 10 to 11 d since the start of production and reinjection. The tracer concentration in the reinjected fluid was maintained at 10 ppm.

The numerical simulations were performed using the code developed by Matsumoto (2020). The code based on the finite difference method can develop a single-phase and non-isothermal discrete-fracture network model, including conductive heat transfer in the direction perpendicular to each fracture. The pressure responses at the flowing wells were accurately simulated using highly refined local grids that accurately simulated the steep pressure variations in the vicinity of the flowing wells. To effectively reduce numerical dispersion, the flows of the tracer were simulated using the constrained interpolation profile method (Yabe et al., 1991; Yabe & Aoki, 1991). The thermodynamic properties and viscosity of water were computed using the empirical equation developed by the IAPWS (IAPWS, 1997, 2007). Using this code, a two-dimensional Cartesian spatial grid was defined for each planar permeable structure (Figure 7). The grid size of the vertical faults in the vertical direction was 100 m. In particular, the horizontal direction varied depending on the region. In the inner region (3.0 km from the reinjection well toward both sides), the grid size was uniform at 100 m. In the outer regions, the grid size expanded exponentially from 100 m to 250 km. Moreover, the grid size for the

horizontal permeable structure (Figure 7a) in the direction perpendicular to the vertical faults was 100 m. In contrast, the grid size for the unknown porous layer (Figure 7b) in the direction perpendicular to the vertical fault varied from 100 m to 1.17 km at most, depending on the dimension  $L$ . For both the horizontal permeable structure (Figure 7a) and the unknown porous layer (Figure 7b) in the direction parallel to the vertical faults, the grid sizes were identical to those for the vertical faults. The temporal grid size expanded exponentially from  $6.61 \times 10^{-8}$  s after four days from starting production and reinjection, followed by a constant value of  $4.74 \times 10^{-1}$  h.



**Figure 7.** Reservoir simulation models. The known vertical fault is connected with (a) unknown faults in parallel or (b) an unknown porous layer. The extremely large horizontal dimension of 2000 km represents infinite-acting boundary conditions. The upper and lower boundaries as well as the boundary along the unknown porous layer on the opposite side of the known fault are impermeable. Under a natural state, the reservoir pressure follows a hydrostatic pressure distribution of 12 MPa at 0.4 km from the upper boundary. The specific enthalpy is uniform at  $1085.8 \text{ kJ kg}^{-1}$  (i.e., approximately uniform temperature distribution at  $250^\circ\text{C}$ ). The physical properties of the faults and porous layer are uniform and constant. The permeability–thickness product is  $1.0 \times 10^{-11} \text{ m}^3$ . Porosity–thickness product is  $2.0 \times 10^{-1} \text{ m}$ , except the unknown

porous layer that is assumed as 1.0 m. The dispersion coefficient for the tracer is  $1.0 \times 10^{-2} \text{ m}^2 \text{ s}^{-1}$ .

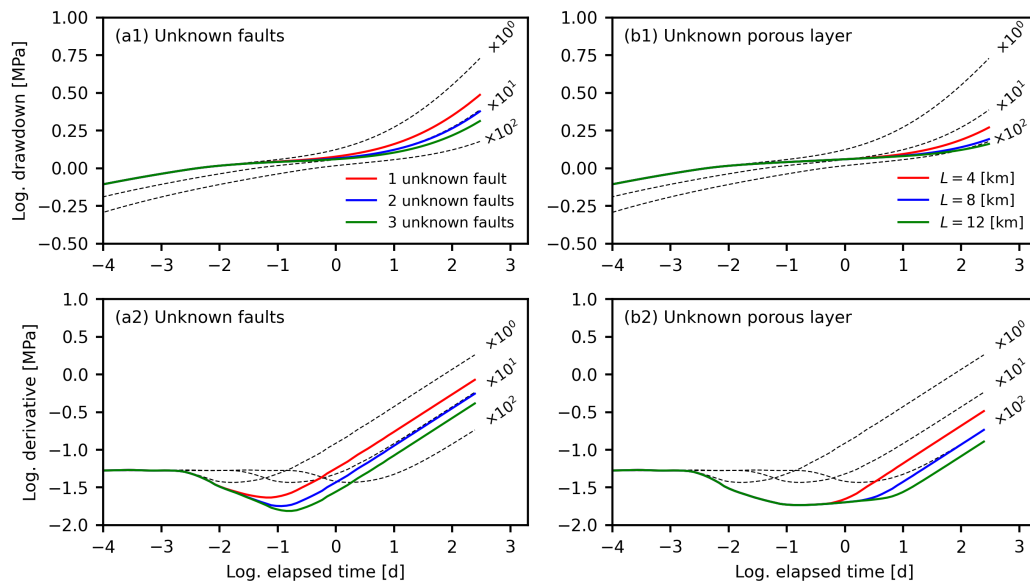
## 4.2 Results and Discussion

The pressure responses at the production well for both the models are depicted in Figure 8, assuming the unknown faults and porous layer in terms of drawdown  $\Delta P$  and its derivative  $\partial \Delta P / \partial (\ln t) = t \partial \Delta P / \partial t$ , where  $t$  denotes the time elapsed since the start of production and reinjection. As conventionally adopted in the well test analysis (Houzé et al., 2012), the pressure responses can be interpreted as a superposition of those for two problems: production and reinjection at the same rate of  $160 \text{ t h}^{-1}$  and production at a rate of  $40 \text{ t h}^{-1}$  without reinjection. As observed, the pressure response component obeying the former problem converged to a steady state that depends solely on transmissivity. In contrast, the latter problem represents production at a constant rate that generates a linear flow depending solely on the product of the transmissivity and storativity (Gringarten et al., 1974). Upon combining these two problems, we can ensure that the pressure response independently relies on transmissivity and storativity.

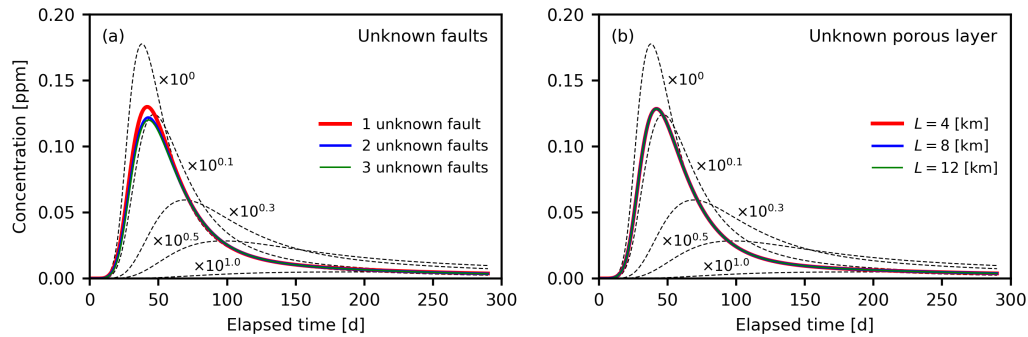
The variations of the apparent porosity–thickness product of the known fault (i.e., aperture of an equivalent planar fracture) after connection to the unknown faults or porous layer are depicted in Figure 8. After coinciding with the original value of the known fault at the initial time interval, the apparent porosity–thickness product gradually increased and converged to a specific value depending on the conditions of the unknown permeable structures. Referring to the straight lines with a slope of 0.5 determined from the derivative, the predominance of the linear flow can be observed (Gringarten et al., 1974). According to the conditions of the unknown permeable structures, the apparent porosity–thickness product varied by orders of magnitude. For the model assuming one to three unknown faults, the apparent porosity–thickness products were  $10^{0.668}$  ( $= 4.66$ ),  $10^{1.04}$  ( $= 11.0$ ), and  $10^{1.30}$  ( $= 19.8$ ) times larger than the original value, respectively. Assuming an unknown porous layer with dimensions of 4, 8, and 12 km, the apparent porosity–thickness products were  $10^{1.50}$  ( $= 31.8$ ),  $10^{2.00}$  ( $= 100$ ), and  $10^{2.31}$  ( $= 205$ ) times larger, respectively.

Similar to the pressure responses, the tracer concentration responses for both the models are illustrated in Figure 9 along with the reference responses. The tracer concentration responses indicated an apparent porosity–thickness product that was  $\sim 10^{0.1}$  ( $= 1.26$ ) times larger than the

original value of the known fault, with a low dependence on the conditions of the unknown permeable structures. This slightly larger value of the apparent porosity–thickness product resulted from the transfer of the tracer via dispersion from the known fault to the unknown permeable structures. For simplicity, the dispersion coefficient in this numerical model was assumed to be constant, although certain sophisticated models often assume smaller values with lower flow velocities (Bear, 1972, 1979). In case of the predominant flow from the reinjection well to the production well only through the known fault, the transfer of the tracer from the known fault to the unknown permeable structures was potentially overestimated. In case of adopting the values in this study, the apparent porosity–thickness products derived from the tracer concentration responses could adequately reproduce the original value of the known fault, regardless of the unknown permeable structures. Based on the tracer concentration responses, the apparent variation in the porosity–thickness product was orders of magnitude less than that derived from the pressure responses depending on the condition of the unknown permeable structures. Thus, the results of the numerical reservoir simulations signify that the deviation of a three-dimensional reservoir structure from an assumed single planar reservoir (i.e., known fault) can be quantified based on the difference between the apertures of equivalent planar fractures (i.e., apparent porosity–thickness products) with respect to pressure and tracer concentration responses, as described in Section 2.



**Figure 8.** Pressure responses at production well after the initiation of production and reinjection for several conditions of unknown permeable structures: variations in drawdown and its derivative assuming one to three unknown faults (a1, a2) and those assuming an unknown porous layer with a dimension of 4 to 12 km (b1, b2). The responses associated with the unknown permeable structures (solid lines) were superimposed on the reference responses, assuming only the known fault with the original porosity–thickness product value, 0.2 m, multiplied by three factors (broken lines).



**Figure 9.** Tracer concentration responses at production well after injection, ten days after starting production and reinjection, for several conditions of the unknown permeable structures: one to three unknown faults (a1, a2) and an unknown porous layer with a dimension of 4 to 12 km (b1, b2). The responses associated with the unknown permeable structures (solid lines) are superimposed on the reference responses assuming the known fault only with the original porosity–thickness product value, 0.2 m, multiplied by five factors (broken lines).

## 5 Discussion

### 5.1 Relationship between Apparent and Actual Pore Volumes

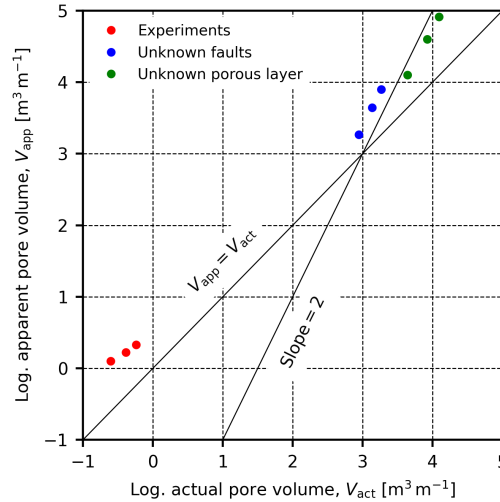
The connection of the unknown buffer tank or permeable structures increased the apparent volumetric capacity of the known tube or fault, respectively, which were estimated from the pressure responses. We then investigated the relationship between the apparent and actual pore volumes (Figure 10). The actual pore volume for the experiments described in Section 3 was determined by estimating the effective volumetric capacity of the buffer tank,  $V' = \bar{c}V/c_w$ , filled with water. The mixture of water and air in the buffer tank bears a volume  $V$  and bulk compressibility  $\bar{c}$ , and  $c_w$  denotes the compressibility of water. To compute the effective

553 volume  $V'$ , the thermodynamic properties of water and air were referred from IAPWS (2007) and  
554 Lemmon et al. (2000), respectively. The volumetric capacity of the tube connecting the buffer  
555 tank and horizontal tube was considered as well. The actual pore volume per unit length was  
556 determined by dividing the total effective volume by the length of the horizontal tube, including  
557 the buffer tank and tubes. The actual pore volume per unit length for the reservoir simulation  
558 models was determined by dividing the total pore volume, including the known fault and  
559 unknown permeable structures (i.e., unknown faults or porous layer), with the length of the  
560 reservoir in the horizontal direction along the known fault. In Figure 10, the apparent pore  
561 volume estimated from the pressure responses were compared with the actual pore volume for  
562 both the laboratory experiments and reservoir simulations described in Sections 3 and 4.

563         Notably, the apparent pore volume tended to be larger than the actual pore volume  
564 (Figure 10), implying that the aperture of an equivalent planar fracture referring to pressure  
565 responses during pressure transient tests included excess pore volume. The relationship between  
566 the apparent and actual pore volumes determined from the reservoir simulations exhibited  
567 straight lines with a slope of 2.0. This indicated that the apparent pore volume was proportional  
568 to the square of the actual pore volume. This specific feature provides an insightful guide to  
569 reveal the mechanism generating excess pore volume. For the reservoir simulations described in  
570 Section 4, the distance between the known fault and the farthest point in the unknown permeable  
571 structure in the direction perpendicular to the known fault increased proportionally with the  
572 actual pore volume of the unknown permeable structure. After the propagation of pressure  
573 variation caused by the production and reinjection reached the farthest point, the pressure  
574 response at the production well transitioned to that representing the linear flow, i.e., a straight  
575 line with a slope of 0.5 (Figure 8a2 and 8b2). As clarified by the radius of investigation in the  
576 well test analysis (Houzé et al., 2012), the time elapsed is proportional to the square of the  
577 distance reached by the propagating pressure variation from a source. The reference lines  
578 described in Figure 8a2 and 8b2 indicate that the apparent porosity–thickness product (i.e.,  
579 apparent pore volume) increased proportionally with the delay, as the straight line appeared.  
580 Thus, the apparent pore volume was proportional to the square of the actual pore volume (Figure  
581 10). From a general perspective, the excess pore volume is generated by the delay in pressure  
582 responses caused by the propagation of pressure variations in the reservoir, independent of  
583 reservoir geometry. The relationship between the apparent and actual pore volumes determined



from laboratory experiments did not exhibit a straight line with a slope of 2.0, because the distance between the horizontal tube and the farthest point in the buffer tank did not vary with the actual pore volume.



**Figure 10.** Relationship between apparent and actual pore volumes derived from laboratory experiments (red circles) in Section 3 and reservoir simulations (blue and green circles) in Section 4.

## 5.2 Potential Applications to Geothermal Exploration and Development

Let us consider a potential application of the concept, validated in this study, to explorational and developmental project at a geothermal prospect associated with a heterogeneous pore distribution generated by multiple faults. As such, the typical procedure for exploration and development has been established in several textbooks (DiPippo, 2016; Grant & Bixley, 2011): ground surveys based on several geosciences, followed by explorational drilling, production testing, including pressure transient and tracer testing, and reservoir modeling. The proposed concept can be applied to interpret the data obtained from pressure transient and tracer tests performed during production. This enables us to quantitatively assess the existence of permeable structures that have not been considered in the reservoir simulation model. The current model is three-dimensional, practically sophisticated, and constructed based on the results of previous surveys, drilling, and tests. As such, an accurate prediction of unknown permeable structures substantiates further surveys and drilling. This assessment will provide an insightful guide for decision-making, such as advancing to the developmental phase, continuing

exploration, or suspending a project. The possible dimensions and locations of unknown permeable structures can be estimated by constructing a reservoir simulation model, including assumed unknown permeable structures, to match the apertures of equivalent planar fractures based on pressure and tracer concentration responses. To more realistically render the model, the results of geological and geophysical surveys and their interpretations provide useful references for constructing and brushing up the model. Adequate observation and understanding of thermodynamic reservoir conditions are crucial because the bulk compressibility of water increases by four orders of magnitude owing to boiling (Grant & Bixley, 2011), which will potentially lead to an overestimation of the aperture of an equivalent planar fracture referring to pressure responses.

Furthermore, the proposed concept enables us to assess the accuracy of a reservoir simulation model at a relatively small scale. Despite several sophisticated and state-of-the-art modeling techniques (Viswanathan et al., 2022) and progress in computational capacity, we cannot avoid simplifying field observations to a greater or lesser extent during modeling. Let us assume that a field-scale model contains a permeable plane representing a fault confirmed from explorational wells (Figure 7). Even if multiple minor lost-circulation depths and/or drilling breaks exist at a considerable depth interval associated with the main lost-circulation depth, we often expressed a fault using a single plane for simplicity. Referring to the concept as well as the pressure and tracer concentration responses observed during pressure transient and tracer tests, respectively, we will be able to assess the accuracy of the model in expressing the actual permeable structure. If the aperture of an equivalent planar fracture referring to the pressure responses coincides with that referring to the tracer concentration responses, the fault comprises a single planar fracture. Otherwise, minor permeable structures are associated with or intersect the main fault. Remarkably, the application of the concept does not require special methods, techniques, or tools for field tests or modeling. Thus, this concept can be immediately applied to existing observational data obtained from conventional testing methods, adopting conventional reservoir simulators capable of simulating pressure and tracer concentration responses.

## 6 Conclusions

This study proposed a novel concept to quantify the deviation of the heterogeneous pore distribution associated with branches and dead ends from a planar distribution. At a larger scale,

the deviation of a three-dimensional reservoir structure from a planar reservoir can be quantified using this concept. Primarily, the difference between the apertures of the equivalent planar fractures estimated from the pressure and tracer concentration responses during the pressure transient and tracer tests, respectively, forms the key parameter of evaluation. In principle, this concept is based on the variations between physical mechanisms in a heterogeneous pore distribution. The pressure responses obey the diffusion problem, whereas the tracer concentration responses obey the advection–dispersion problem.

The proposed concept was validated through laboratory experiments using quasi-Hagen–Poiseuille flow in a urethane tube. A buffer tank mimicking a branch and dead end was connected to the tube midpoint. The apparent inner diameter of the tube, determined from the pressure responses, varied up to 425 times larger than the actual value, depending on the buffer tank condition. Nonetheless, the tracer concentration responses could be independently reproduced by assuming an approximate actual inner diameter, regardless of the buffer tank condition. The difference between the apparent and actual inner diameters varied with the effective volume of the buffer tank, and the deviation from a single tube was successfully quantified.

Furthermore, the concept was validated on a larger scale using reservoir simulations, assuming a known fault and unknown permeable structures (i.e., unknown faults or a porous layer). Specifically, the pressure and tracer concentration responses were simulated assuming production and reinjection wells intersecting only the known fault. The apparent porosity–thickness product (i.e., aperture of an equivalent planar fracture) determined from the pressure responses varied up to 205 times larger than the actual value of the known fault, depending on the condition of the unknown permeable structures. Overall, the tracer concentration responses could be reproduced by assuming an approximate actual porosity–thickness product, regardless of the condition of the unknown permeable structures. The difference between the apparent and actual porosity–thickness products varied with the volume of the unknown permeable structures and successfully quantified the deviation from a single fault.

The apparent pore volume estimated from the pressure responses tended to be larger than the actual value owing to the delay in pressure responses. In general, a delay is caused when the pressure variation propagates through the pore. This implies that the aperture of an equivalent

planar fracture, determined from pressure transient tests, includes an excess pore volume beyond the actual reservoir pore volume. The concept will provide a useful guide for successful explorational and developmental geothermal projects by quantifying the existence of undiscovered permeable structures in a reservoir simulation model. In addition, this concept provides a scale for assessing a reservoir simulation model in accurate expression of an actual heterogeneous permeable structure.

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## Open Research

The data, including the image files, experimental results, numerical input/output data, and source codes, presented in each figure and table are available at the repository provided by HYDROSHARE via <http://www.hydroshare.org/resource/3b66eb5ff9be4dc19d5e726bf4e08cc7> (Matsumoto, 2022).

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